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Design criteria for infiltration



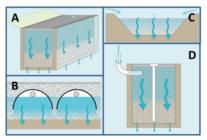
This page provides a discussion of design elements and design steps for **infiltration** (https://stormwater. pca.state.mn.us/index.php?title=Stormwater_infiltration_Best_Management_Practices) practices. These practices include infiltration trench, **infiltration basin** (https://stormwater.pca.state.mn.us/index.php?titl e=Infiltration), dry wells, and underground infiltration practices, although many of the design

guidelines can be applied to other practices where infiltration can occur (http://stormwater.pca.state.mn.us/index.php/Sto rmwater_infiltration_Best_Management_Practices). For a descriptions of different types of infiltration practices to which these design guidelines apply, link here (https://stormwater.pca.state.mn.us/index.php?title=BMPs_for_stormwater_infilt ration).

Green Infrastructure: Infiltration practices can be an important tool for retention and detention of stormwater runoff. Depending on design and construction, infiltration practices may provide additional benefits, including cleaner air, carbon sequestration, improved biological habitat, and aesthetic value.

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Schematic illustrating A)infiltration trench, B) underground infiltration, C) infiltration basin, and D) dry well.

Permit requirements

Link here (https://stormwater.pca.state.mn.us/index.php?title=MN_CSW_Permit_Section_16_Infiltration_Systems) for requirements of the Construction Stormwater General Permit (https://stormwater.pca.state.mn.us/index.php?title=Construction_stormwater_program) for infiltration systems

Terminology

The following terminology is used throughout this page.

Warning: REQUIRED - Indicates requirements stipulated by the MPCA Construction General Permit (http://stormwater.pca.state.mn.us/index.php/Construction_stormwate r_permit) (CGP) or other consistently applicable regulations

REQUIRED - Indicates standards stipulated by the CGP or other consistently applicable regulations.

HIGHLY RECOMMENDED - Indicates guidance or other information that is extremely beneficial or necessary, but not specifically required by the MPCA CGP.

RECOMMENDED - Indicates guidance or other information that is helpful but not critical.

Details and CADD images

- CADD dwg file (http://stormwater.pca.state.mn.us/index.php/File:Infiltration basin plan and profile.dwg) for infiltration basin plan and profile
- CADD images
 - Infiltration basin plan and profile: File:Infiltration Basin INFILTRATION BASIN PLAN & PROFILE (1).pdf
 - Infiltration subsurface layout: File:Infiltration Subsurface Layout2 (1).pdf
 - Infiltration trench section: File:INFILTRATION TRENCH INFILTRATION TRENCH SECTION (1).pdf
- Details for underground systems: Underground systems are proprietary devices. Detail varies between each practice. We therefore include links to examples of
 commonly used devices. We acknowledge there are many more of these devices and reference to any specific commercial product, process, or service by trade name,
 trademark, service mark, manufacturer, or otherwise does not constitute or imply endorsement, recommendation, or favoring by the Minnesota Pollution Control
 Agency.
 - ChamberMaxx Retention (http://www.conteches.com/DesktopModules/Bring2mind/DMX/Download.aspx?command=core_download&entryid=2767&languag e=en-US&PortalId=0&TabId=144)
 - Terre Arch (http://www.conteches.com/DesktopModules/Bring2mind/DMX/Download.aspx?command=core_download&entryid=13557&language=en-US&PortalId=0&TabId=144)
 - Triton Stormwater Solutions Chamber System (https://www.tritonsws.com/)
 - StormTrap Singletrap example project (http://stormtrap.com/wp-content/uploads/2016/04/Sample-Drawing-SingleTrap-Infiltration.pdf) and StormTrap Doubletrap example project (http://stormtrap.com/wp-content/uploads/2016/04/Sample-Drawing-DoubleTrap-Infiltration.pdf)
 - Stormtech systems (http://www.stormtech.com/resources/downloads.html)
 - Contech systems (http://www.conteches.com/DesktopModules/Bring2mind/DMX/Download.aspx?command=core_download&entryid=13641&language=en-U S&PortalId=0&TabId=144)

Caution: Reference to any specific commercial product, process, or service by trade name, trademark, service mark, manufacturer, or otherwise does not constitute or imply endorsement, recommendation, or favoring by the Minnesota Pollution Control Agency.

Design phase maintenance considerations

Caution: Maintenance considerations are an important component of design

Implicit in the design guidance is the fact that many design elements of infiltration systems can minimize the maintenance burden and maintain pollutant removal efficiency. Key examples include

- limiting contributing drainage area (https://stormwater.pca.state.mn.us/index.php?title=Contributing drainage area to stormwater BMPs);
- providing easy site access (REQUIRED);
- providing pretreatment (https://stormwater.pca.state.mn.us/index.php?title=Pretreatment) (REQUIRED); and
- utilizing native species plantings (see Plants for Stormwater Design (http://www.pca.state.mn.us/publications/manuals/stormwaterplants.html)).

For more information on design information for individual infiltration practices, link here (http://stormwater.pca.state.mn.us/index.php/Category:Design_criteria).

Major design elements - Physical feasibility initial check

Before deciding to use an infiltration practice for stormwater management, it is helpful to consider several items that bear on the feasibility of using such a device at a given location. This section describes considerations in making an initial judgment as to whether or not an infiltration practice is the appropriate BMP for the site. The following links provide additional information on specific constraints to infiltration.

- Karst
- Shallow soils and shallow depth to bedrock
- Shallow groundwater
- Soils with low infiltration capacity
- Separation distances
- Potential stormwater hotspots
- Wellhead protection
- Contaminated soils and groundwater
- Procedures for investigating sites with potential constraints (http://stormwater.pca.state.mn.us/index.php/Procedures_for_investigating_sites_with_potential_constraint s_on_stormwater_infiltration)

Contributing drainage area

Design criteria for infiltration - Minnesota Stormwater Manual

Contributing drainage area is defined as the total area, including pervious and impervious surfaces, contributing to a **best management practice** (BMP). It is assumed that in most cases, with the exception of **green roofs (https://stormwater.pca.state.mn.us/index.php?title=Green_roofs)** and many **permeable pavement (https://stormwater.pc ca.state.mn.us/index.php?title=Permeable_pavement)** systems, impervious surfaces will constitute more than 50 percent of the contributing area to the BMP and that most of this impervious is **directly connected impervious**. The recommended contributing area to a BMP may be modified for the following conditions.

- The recommended contributing area may be increased if pervious surfaces constitute the majority of the contributing area and soils are hydrologic soil group (https://stormwater.pca.state.mn.us/index.php?title=Design_infiltration_rates) (HSG) A or B
- The recommended contributing area should be decreased if impervious surfaces contribute more than 80 percent of the contributing area or if the entire impervious surface is directly connected and routed to the BMP
- The recommended contributing area should be decreased or may be increased based on pollutant loading (decrease with higher pollutant loads)

Runoff coefficients (https://stormwater.pca.state.mn.us/index.php?title=Runoff_coefficients_for_5_to_10_year_storms) may be calculated for an area contributing to a BMP. Runoff coefficients greater than about 0.55 are typical of urban areas having 50 percent or more impervious surface. Typical runoff coefficients are shown on these pages ([1] (https://stormwater.pca.state.mn.us/index.php/Runoff_coefficients_for_5_to_10_year_storms), [2] (https://stormwater.pca.state.mn.us/index.php/Runoff_coefficients_coefficients_28Rv.29). To see how runoff curve number is associated with impervious percentages, see Table 2-2a, page 17, at this link (https://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/stelprdb1044171.pdf).

It is *HIGHLY RECOMMENDED* that the following infiltration practices be designed with the indicated maximum drainage areas. See the table below for recommended contributing drainage areas for all infiltration BMPs.

- Dry well 1 acre or less; rooftop only
- Infiltration Trench 5 acres or less
- Underground Infiltration System 10 acres or less
- Infiltration Basin 50 acres or less. Contributing drainage areas should have a maximum 5:1 ratio of impervious area to infiltration area.

Infiltration practices must meet the required 48 hour drawdown time and must be sized in order to allow for adequate maintenance without increasing compaction. Information on recommended contributing drainage areas for all infiltration practices is shown in the following table.

Stormwater infiltration BMPs - contributing drainage area

Link to this table

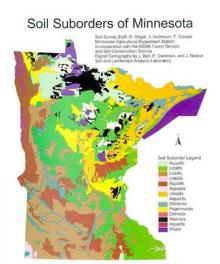
Stormwater BMP	Recommended contributing area	Notes
Infiltration Basin	50 acres or less	A natural or constructed impoundment that captures, temporarily stores and infiltrates the design volume of water into the surrounding naturally permeable soil over several days. In the case of a constructed basin, the impoundment is created by excavation or embankment.
Bioinfiltration Basin	5 acres or less	Bioinfiltration basins must meet the required 48 hour drawdown time and must be sized in order to allow for adequate maintenance. It is HIGHLY RECOMMENDED that bioinfiltration basins be designed to prevent high levels of bounce as submerging vegetation may inhibit plant growth. A maximum wet storage depth of 1.5 feet is HIGHLY RECOMMENDED.
Infiltration Trench	5 acres or less	
Dry Well Synonym: Infiltration Tube, French Drain, Soak- Away Pits, Soak Holes	1 acre or less (rooftop only)	
Underground Infiltration	10 acres or less	Though feasible, larger underground infiltration systems may cause groundwater contamination as water is not able to infiltrate through a surface cover. In addition, wind flocculation, UV degradation, and bacterial degradation, which provide additional treatment in surface systems, do not occur in underground systems. Because performance research is lacking for larger features, it is HIGHLY RECOMMENDED that the contributing drainage area to a single device not exceed 10 acres.
Dry Swale with Check Dams	5 acres or less	
Permeable Pavement	It is RECOMMENDED that external contributing drainage area not exceed the surface area of the permeable pavement. It is HIGHLY RECOMMENDED that external contributing drainage area not exceed twice the surface area of the permeable pavement	It is RECOMMENDED that external drainage area be as close to 100% impervious as possible. Field experience has shown that drainage area (pervious or impervious) can contribute particulates to the permeable pavement and lead to clogging. Therefore, sediment source control and/or pretreatment should be used to control sediment run-on to the permeable pavement section.
Tree Trench/Tree Box	up to 0.25 acres per tree	
		*8_INFILTRATION_v1-9_03012011.pdf), North Carolina (https://deq.nc.gov/about/divisions/energy-mineral-land-resources/energy-mineral-land-permit-guidance/st waterManagementDesignandGuidanceManual.aspx), Maine (http://www.maine.gov/dep/land/stormwater/stormwaterbmps/), Lake Tahoe (http://tahoebmp.org/bmpha

ormwater-bmp-manual), West Virginia (http://www.dep.wv.gov/WWE/Programs/stormwater/MS4/Pages/Stormwater/ManagementDesignandGuidanceManual.aspx), Maine (http://www.maine.gov/dep/land/stormwater/storm

Site Topography and Slopes

Unless slope stability calculations demonstrate otherwise (see [3] (http://people.eng.unimelb.edu.au/stsy/geomechanics_text/Ch11_Slope.pdf), [4] (http://www.gf.uns.ac.rs/~ zbornik/doc/ZR25.51.pdf)), it is *HIGHLY RECOMMENDED* that infiltration practices be located a minimum horizontal distance of 200 feet from down-gradient slopes greater than 20 percent, and that slopes in contributing drainage areas be limited to 15 percent.

Soils



Statewide map of soil suborders. While soil infiltration rates vary widely within a soil order or suborder, infiltration rates tend to be lower on mollisols, histisols and vertisols compared to entisols, inceptisols, spodosols, and alfisols. Source: University of Minnesota (https://extension.umn.edu/soil-managementand-health/soil-orders-and-suborders-minneso ta) It is *HIGHLY RECOMMENDED* that native soils in proposed infiltration areas have a minimum **infiltration rate** of 0.2 inches per hour (typically **hydrologic soil group (https://stormwater.pca.state.mn.us/index.php?title=Design_infiltr ation_rates)** A, B and C soils). Initially, soil infiltration rates can be estimated from NRCS (https://websoilsurvey.sc.ego v.usda.gov/App/HomePage.htm) soil data, and confirmed with an on-site infiltration evaluation or geotechnical investigation (see Step 5 of the Design procedures (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_infil tration#Step_5._Determine_infiltration_type_and_size_practice) section for investigation procedures). It is *HIGHLY RECOMMENDED* that native soils have silt/clay contents less than 40 percent and clay content less than 20 percent, and that infiltration practices not be situated in **fill** soils.

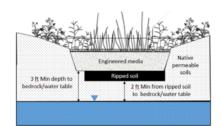
Warning: It is *REQUIRED* that impervious area construction be completed and pervious areas established with dense and healthy vegetation prior to introduction of stormwater into an infiltration practice.

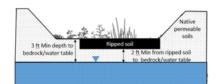
Depth to groundwater table and bedrock

Warning: A separation distance of 3 feet is REQUIRED between the bottom of the infiltration practice and the elevation of the seasonally high water table (saturated soil) or top of bedrock (i.e. there must be a minimum of 3 feet of undisturbed soil beneath the infiltration practice and the seasonally high water table or top of bedrock). See also Step 7 (http://stormwater.pca.state.mn.us/index.p hp/Design_criteria_for_infiltration#Step_7._Perform_ground-water_ mounding_analysis) under the Design procedures section.

Note that if underlying soils are ripped (https://stormwater.pca.state. mn.us/index.php?title=Alleviating_compaction_from_construction_ activities#Soil_ripping) to alleviate compaction (https://stormwater. pca.state.mn.us/index.php/Alleviating_compaction_from_constructi on_activities), the requirement is a 2 foot minimum between the bottom of the ripped zone and a 3 foot minimum from the bottom of the infiltration practice. If there is only a 3 foot separation distance between the bottom of the infiltration practice and the elevation of the seasonally high water table (https://stormwater.pca.state.mn.us/in dex.php?title=Shallow_groundwater) or bedrock (https://stormwater. pca.state.mn.us/index.php?title=Shallow_soils_and_shallow_depth_

to_bedrock), limit ripping depth to 12 inches. See the alleviating compaction (http://stormwater.pca.state.mn.us/index.ph p/Alleviating_compaction_from_construction_activities) and soil ripping (http://stormwater.pca.state.mn.us/index.php/A lleviating_compaction_from_construction_activities#Soil_ripping) webpages for more detail on compaction prevention.





Schematic illustrating separation distance from bottom of infiltration BMP and soil ripped zones to water table or top of bedrock

Site Location / Minimum Setbacks

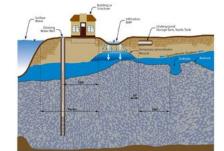
The following table summarizes horizontal and vertical setback distances for required and recommended minimum distances from an infiltration practice to an above-ground or underground structure. It will be necessary to consult local ordinances for further guidance on siting infiltration practices.

Warning: The minimum setback for public water supplies in the table below are *REQUIRED* by the Minnesota Department of Health for the design and location of infiltration practices. It will be necessary to consult local ordinances for further guidance on siting infiltration practices.

Required and recommended minimum vertical and horizontal separation distances. This represents the minimum distance from the infiltration practice to the structure of concern. If the structure is above-ground, the distance is measured from the edge of the BMP to the structure. If the structure is underground, the vertical separation distance represents the distance from the point of infiltration through the bottom of the system to the structure, while the horizontal separation (often called setback) distance is the shortest distance from the edge of the system to the structure.

Link to this table

	Structure	Distance (feet)	Requirement or recommendation	Note(s)
Vertical	Saturated soil (https://stormwater.pca.st ate.mn.us/index.php?title=Shallow_gro undwater)		Requirement ¹	
vertical	Bedrock (https://stormwater.pca.state. mn.us/index.php?title=Shallow_soils_a nd_shallow_depth_to_bedrock)	3	Requirement ¹	
Horizontal	Public supply well (https://stormwater. pca.state.mn.us/index.php?title=Storm water_and_wellhead_protection)	100 for sensitive wells; 50 for others ³	Requirement	
	Building/structure/property line ²	10	Recommended	



Schematic showing some horizontal and vertical separation distances from an infiltration BMP. A separation distance may be required, such as with a drinking water well, or recommended, as with an underground tank. (Source: CDM Smith) Not to scale.

Structure	Distance (feet)	Requirement or recommendation	Note(s)
Surface water	none unless local requirements exist		If nearby stream is impaired for chloride, see [5] (http://stormwa ter.pca.state.mn.us/index.php/St ormwater_infiltration_and_setb ack_%28separation%29_distanc es#Surface_waters)
Septic system	35	Recommended	
Contaminated soil/groundwater (http s://stormwater.pca.state.mn.us/index.ph p?title=Stormwater_infiltration_and_co ntaminated_soils_and_groundwater)			No specific distance. Infiltration must not mobilize contaminants.
Slope	200	Recommended	from toe of slope $\geq 20\%$
Karst	1000 up- gradient 100 down- gradient	Requirement ¹	active karst (https://stormwat er.pca.state.mn.us/index.php?t itle=Karst)

¹ Required under the Construction Stormwater General Permit

² Minimum with slopes directed away from the building

³If treating an average of 10,000 gallons per day; otherwise separation distance is 300 feet

Karst

It is *HIGHLY RECOMMENDED* that infiltration practices not be used in **active karst (https://stormwater.pca.state.mn.us/index.php?title=Karst)** formations without adequate geotechnical testing (http://stormwater.pca.state.mn.us/index.php/Karst#How_to_investigate_for_karst_on_a_site).

Warning: The Construction Stormwater permit prohibits permittees from constructing infiltration systems in areas within 1,000 feet upgradient or 100 feet downgradient of active karst features

Wellhead Protection Areas

See stormwater and **wellhead protection** (https://stormwater.pca.state.mn.us/index.php?title=Stormwater_and_wellhead_protection) for guidance and recommendations for determining the appropriateness of infiltrating stormwater in a Drinking Water Supply Management Area (http://stormwater.pca.state.mn.us/index.php/Stormwater_and_well head_protection) (DWSMA). For more information on source water protection see Minnesota Department of Health (http://www.health.state.mn.us/divs/eh/water/swp/).

Major design elements - Practice and site considerations

If physical attributes of a site do not prohibit infiltration, there are several considerations for the infiltration practice and site.

Conveyance

It is *HIGHLY RECOMMENDED* that a flow splitter (https://megamanual.geosyntec.com/npsmanual/flowsplitter.aspx) or diversion structure be provided to divert the **Water Quality Volume** (https://stormwater.pca.state.mn.us/index.php?title= Water_quality_criteria) (V_{wq}) to the infiltration practice and allow larger flows to **bypass** the practice unless the infiltration practice is sized to retain larger volumes (Channel protection criteria (Vcp) (http://stormwater.pca.state.mn.u s/index.php/Channel_protection_criteria_%28Vcp%29), Overbank flood protection criteria (Vp10) (http://stormwater.pca.state.mn.u s/index.php/Coverbank_flood_protection_criteria_%28Vp10%29) or Extreme flood control criteria (Vp100) (http://stormwater.pca.state.mn.us/index.php/Overbank_flood_protection_criteria_%28Vp10%29)). Where a flow splitter is not used, it is *HIGHLY RECOMMENDED* that contributing drainage areas be limited to the appropriate size given the bmp and an overflow be provided within the practice to pass part of the V_{wq} to a stabilized watercourse or storm drain. It is also *HIGHLY RECOMMENDED* that overflow associated with the V_{p10} or V_{p100} storm (depending on local drainage criteria) be controlled such that velocities are non-erosive at the outlet point (to prevent downstream slope erosion), and that when discharge flows exceed 3 cubic feet per second, the designer evaluate the potential for erosion to stabilized areas and infiltration facilities.



Photo of a Infiltration trench in Lino Lakes. This site illustrates how site aesthetics and landscaping are considered in the BMP design.

An infiltration device can be designed to accommodate a concentrated influent flow; however, an energy dissipater and/or level spreader may be needed. See the pretreatment section for more information on pretreatment devices.

Underdrains

Perforated elevated underdrains are sometimes used to facilitate infiltration. If an infiltration system does not have an underdrain, it should be designed with dewatering provisions in the event of failure. This can be done with underdrain pipe systems that can be pumped out or allowed to gravity drain to the surface.

The following are RECOMMENDED for infiltration practices with underdrains.

- The minimum pipe diameter is 4 inches.
- Install 2 or more underdrains for each infiltration system in case one clogs. At a minimum provide one underdrain for every 1,000 square feet of surface area.
 Include at least 2 observation/cleanouts for each underdrain, one at the upstream end and one at the downstream end. Cleanouts should be at least 4 inches diameter
- Construct underdrains with Schedule 40 or SDR 35 smooth wall PVC pipe (https://www.homestratosphere.com/pvc-pipe-and-fittings-sizes/).
- Install underdrains with a minimum slope of 0.5 percent, particularly in hydrologic soil group (https://stormwater.pca.state.mn.us/index.php?title=Design_infiltra tion_rates) D soils (Note: to utilize Manning's equation (http://www.fsl.orst.edu/geowater/FX3/help/8_Hydraulic_Reference/Manning_s_Equation.htm#:~:text=The%

20 Mannings% 20 equation% 20 is% 20 an, flow% 20 area% 20 and% 20 channel% 20 slope. & text=Under% 20 the% 20 assumption% 20 of% 20 uniform, and% 20 the% 20 surface% 20 slope.) the slope must be greater than 0).

- Include a utility trace wire for all buried piping.
- For underdrains that daylight on grade, include a marking stake and animal guard;
- For each underdrain have an accessible knife gate valve on its outlet to allow the option of operating the system as either an infiltration (https://stormwater.pca.state.mn.us/index.php?title=Stormwater_infiltration_Best_Management_Practices) system, filtration (https://stormwater.pca.state.mn.us/index.php?title=Filtration) system, or both. The valve should enable the ability to make adjustments to the discharge flow so the sum of the infiltration rate plus the underdrain discharge rate equal a 48 hour drawdown time.
- Perforations should be 3/8 inches. Use solid sections of non-perforated PVC piping and watertight joints wherever the underdrain system passes below berms, down steep slopes, makes a connection to a drainage structure, or daylights on grade.
- Spacing of collection laterals should be less than 25 feet.
- Underdrain pipes should have a minimum of 3 inches of washed #57 stone (https://www.dirtconnections.com/what-is-57-stone/) above and on each side of the pipe (stone is not required below the pipe). Above the stone, two inches of choking stone is needed to protect the underdrain from blockage.
- Avoid filter fabric .
- Pipe socks may be needed for underdrains imbedded in sand. If pipe socks are used, then use circular knit fabric.

The procedure to size underdrains is typically determined by the project engineer. An example for sizing underdrains is found in the North Carolina Department of Environment and Natural Resources Stormwater BMP Manual (https://deq.nc.gov/about/divisions/energy -mineral-and-land-resources/stormwater/stormwater-program/stormwater-design). Underdrain spacing can be calculated using the following spreadsheet, which utilizes the vanSchilfgaarde Equation. The spreadsheet includes an example calculation. File:Underdrain spacing calculation.xlsx

Pretreatment

Warning: It is *REQUIRED* that some form of pretreatment, such as a plunge pool, sump pit, filter strip, sedimentation basin, grass channel, or a combination of these practices be installed upstream of the infiltration practice.

It is HIGHLY RECOMMENDED that the following pretreatment sizing guidelines be followed:

- Before entering an infiltration practice, stormwater should first enter a pretreatment practice sized to treat a minimum volume of 25 percent of the V_{wq} .
- If the infiltration rate of the native soils exceeds 2 inches per hour a pretreatment practice capable of treating a minimum volume of 50 percent of the V_{wq} should be installed.
- If the infiltration rate of the native soils exceeds 5 inches per hour a pretreatment practice capable of treating a minimum volume of 100 percent of the V_{wq} should be installed.

Alternatively, sizing guidelines based on a target particles size to be captured are found here (https://stormwater.pca.state.mn.us/index.php?title=Pretreatment_sizing_for_bas ins and filters strips).

It is *HIGHLY RECOMMENDED* that pretreatment practices be designed such that exit velocities from the pretreatment systems are non-erosive (less than 3 feet per second) and flows are evenly distributed across the width of the practice (e.g., by using a level spreader).

Caution: - Use low-impact earth moving equipment - DO NOT Overexcavate

For additional information, see the pretreatment section in the manual.

Treatment

Drawdown time and bounce

For information on assessing the performance of a BMP and determining if it meets the required drawdown time, click here (http://stormwater.pca.state.mn.us/index.php/Assessing the performance of infiltration).

Warning: The Construction Stormwater permit states: "The Permittee(s) must design the infiltration/filtration system to discharge the water quality volume routed to the system through the soil surface or filter media within 48 hours or less. Additional flows that cannot be infiltrated or filtered within 48 hours must be routed to bypass the system through a stabilized discharge point. The Permittee(s) must design the infiltration system to provide a means to visually verify that the system is discharging through the soil surface or filter media within 48 hours or less."

Space varies depending on the depth of the practice. Typically, infiltration trenches are 3 to 12 feet deep with a width

less than 25 feet. A dry well is essentially a smaller version of an infiltration trench, consistent with the fact that the

drainage area to an infiltration trench is typically five times greater (or larger) than that of a dry well. Underground

systems have an average depth of 2 to 5 feet depending on the pavement and media thickness. The surface area of all infiltration practices is a function of MPCA's 48-hour drawdown requirement and the infiltration capacity of the

infiltration systems are larger practices that range in depth from approximately 2 to 12 feet. Permeable Pavement

Space occupied

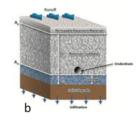
high water level

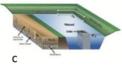
This schematic defines and demonstrates the concept of bounce and drawdown within an infiltration practice. This figure conceptualizes the water quality volume, the high water level, the proposed drawdown depth, and bounce as they relate to an infiltration practice. Click on image to enlarge.

underlying soils. enlarge.
The maximum storage volume statically stored within the infiltration practice must completely drawdown within 48
hours. An emergency spillway and/or backup underdrain should be constructed if the infiltration device is unable to dewater within 48 hours.

Warning: The Construction Stormwater permit states: "The Permittee(s) must design the infiltration/filtration system to discharge the water quality volume routed to the system through the soil surface or filter media within 48 hours or less. Additional flows that cannot be infiltrated or filtered within 48 hours must be routed to bypass the system through a stabilized discharge point. The Permittee(s) must design the infiltration system to provide a means to visually verify that the system is discharging through the soil surface or filter media within 48 hours or less."







Example filtration practices having underdrains: a) biofiltration; b) permeable pavement; c) sand filter. (Source: CDM Smith).

Slope of the infiltration practice

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It is RECOMMENDED that the bottom of all infiltration practices be flat, in order to enable even distribution and infiltration of stormwater. It is RECOMMENDED that the longitudinal slope range only from the ideal 0 percent up to 1 percent, and that lateral slopes be held at 0 percent.

Side slopes

It is HIGHLY RECOMMENDED that the maximum side slope for an infiltration practice is 3:1 (h:v).

Depth

The depth of an infiltration practice is a function of the maximum drawdown time and the design infiltration rate. When the drawdown time for an infiltration system is 48 hours, the total drawdown depth is 78.2 inches for GW and GP Hydrologic Soil Group (HSG) A soils, 38.4 inches for GM and SW (HSG A) soils; 21.6 inches for SM (HSG B) soils; 14.4 inches for loam, silt loam and MH (HSG B) soils; and 9.6 inches for HSG C soils. If field tested rates (https://storm water.pca.state.mn.us/index.php?title=Determining_soil_infiltration_rates) for any soil exceeds the rate for A soils in the manual (1.63 inches per hour), the total drawdown depth (water quality volume depth + bounce) must not exceed 6.5



Schematic showing the effective infiltration area for side slopes less than 3H:1V.

feet. When the drawdown time is 24 hours, the above water quality volume depths are reduced by a factor of 2. Any captured depth (bounce) beyond the water quality volume needs to be removed from the BMP within 48 hours via an emergency spillway designed to overflow at the top of the storage volume and/or a control structure used to limit peak discharge rates.

Warning: The REQUIRED drawdown time for infiltration practices is 48 hours or less, and so the depth of the practice should be determined accordingly. Warning: Groundwater Protection: It is REOUIRED that runoff from potential stormwater hotspots (PSHs) not be infiltrated unless adequate pretreatment has been provided. Infiltration of runoff from confirmed hotspot areas, industrial areas with exposed significant materials, or vehicle fueling and maintenance areas is PROHIBITED.

Aesthetics

Infiltration basins can be effectively integrated into the site planning process, and aesthetically designed as attractive green spaces planted with native vegetation. If vegetation is used, the infiltration practice becomes a bioinfiltration practice. See the Design Criteria for bioinfiltration practices webpage (http://stormwater.pca.state.mn.us/ index.php/Design criteria for bioretention) for more information. Infiltration trenches are less conducive to site aesthetics, but the surface of trenches can be designed with turf cover crops if desired.

Concerning infiltration practices with exposed filter media, keep adjacent vegetation from forming an overhead canopy above infiltration practices, in order to keep leaf litter, fruits, and other vegetative materials from clogging the filter media.

Landscaping

Warning: It is REQUIRED that impervious area construction is completed and pervious areas established with dense and healthy vegetation prior to introduction of stormwater into a vegetated infiltration practice.

Landscaping is critical to the performance and function of vegetated areas of infiltration practices. Therefore, a landscaping plan is HIGHLY RECOMMENDED for vegetated infiltration practices. RECOMMENDED planting guidelines for vegetated practices are as follows.

- Vegetation should be selected based on a specified zone of hydric tolerance. Plants for Stormwater Design -Species selection for the Upper Midwest (http://www.pca.state.mn.us/index.php/water/water-types-and-programs/s tormwater/stormwater-management/plants-for-stormwater-design.html) is a good resource.
- Native plant species should be specified over non-native species. Hardy native species that thrive in our ecosystem without chemical fertilizers and pesticides are the best choices.
- Many vegetated practices feature wild flowers and grasses as well as shrubs and some trees.
- If woody vegetation is placed near inflow locations, it should be kept out of pretreatment (https://stormwater.pca. state.mn.us/index.php?title=Pretreatment) devices and be far enough away to not hamper maintenance of pretreatment devices.
- Trees should not be planted directly overtop of underdrain and may be best located along the perimeter of the practice.
- Salt resistant vegetation should be used in locations with probable adjacent salt application, e.g. roadside, parking lot. etc.
- Plugs, bare root plants or potted plants are RECOMMENDED over seed for herbaceous plants, shrubs, and trees. Erosion control mats (https://stormwater.pca.state.mn.us/index.php?title=Erosion_prevention_practices) pre-vegetated with herbaceous perennial plants are also acceptable. For turf, sod is recommended over seed. (NOTE: Fluctuating water levels following seeding (prior to germination) can cause seed to float and be transported, resulting in bare areas that are more prone to erosion and weed invasion than vegetated areas. Seed is also difficult to establish through mulch (https://stor mwater.pca.state.mn.us/index.php?title=Erosion prevention practices - natural and synthetic mulches), a common surface component of vegetated practices. It may take more than two growing seasons to establish the function and desired aesthetic of mature vegetation via seeding.)
- Vegetated practices should be operated offline for 1 year or, within the first year, until vegetation is established.
- Example target plant coverage includes
 - at least 50 percent of specified vegetation cover at end of the first growing season;
 - at least 90 percent of specified vegetation cover at end of the third growing season;
 - supplement plantings to meet project specifications if cover targets are not met; and
 - tailoring percent coverage targets to project goals and vegetation. For example, percent cover required for turf after 1 growing season would likely be 100 percent, whereas it would likely be lower for other vegetation types.
- Vegetated areas should be integrated into the site planning process, and aesthetic considerations should be taken into account in their siting and design.

Operation and maintenance of vegetated practices is critical to meeting these landscape recommendations and targets. For more information on operation and maintenance, see the section on operation and maintenance of stormwater infiltration practices.

Safety



habitat and aesthetic value, in addition to water quality and quantity benefits.

A well landscaped stormwater practice provides multiple benefits, such as pollinator

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Dry wells, infiltration trenches and subsurface infiltration systems do not pose any major safety hazards. Infiltration basins should have similar side slope considerations as ponds (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_stormwater_ponds#Grading_and_site_layout) and wetlands.

Warning: If a dry well or infiltration trench is greater than five feet deep, it is *REQUIRED* that OSHA health and safety guidelines be followed for safe construction practices.

Additional information on safety for construction sites is available from OSHA (http://www.osha.gov/pls/oshaweb/owadisp.show_document?p_table=STANDARDS&p_id= 10930).

Maximum flow path

Flow path length is important only if high flows are not bypassed. Below are recommendations from other states or localities.

- North Carolina: The geometry of the cell shall be such that width, length, or radius are not less than 10 feet. This is to provide sufficient space for plants.
- Virginia (https://swbmpvwrrc.wp.prod.es.cloud.vt.edu/wp-content/uploads/2017/11/BMP-Spec-No-9_BIORETENTION_v1-9_03012011.pdf): Length of shortest flow
 path to overall length is 0.3 for Level 1 Design and 0.8 for Level 2 Design
- Dakota County Soil and Water Conservation District: Where offline designs are not achievable, bioretention practices shall be designed to route high flows on the
 shortest flow path across the cell to provide the least disturbance and displacement of the Water Quality Volume (https://stormwater.pca.state.mn.us/index.php?title=
 Water_quality_criteria) to be treated. Energy dissipation to avoid high flow velocity turbulence is required.

Use of multiple cells

In comparison to multiple cells, one large bioretention or infiltration cell will often perform just as well as multiple smaller cells if sized and designed appropriately. One large cell is generally less costly than multiple smaller cells. This is due to the simpler geometry and grading requirements of one large cell, as well as a reduction in piping and outlet structures. Multiple smaller cells do however provide greater redundancy, i.e. if one large cell fails, more function is lost than if just one of multiple cells fail. Multiple cells are also more feasible than one large cell in steep terrain (slopes greater than 5 percent), where they can be terraced to match the existing grade. Provided access is maintained to each cell, multiple cells typically results in less and easier maintenance.

Snow considerations

Considering management of snow, the following are recommended.

- Plan a plow path during design phase and tell snowplow operators where to push the snow. Plan trees around (not in) plow path, with a 16 foot minimum between trees.
- Plan for snow storage (both temporary during construction and permanent). Don't plow into raingardens (bioretention) routinely. Raingardens should be a last resort for snow storage (i.e. only for very large snow events as "emergency overflow".
- Snow storage could be, for example, a pretreatment moat around a raingarden, i.e. a forebay for snow melt.

For more information and example photos, see the section on snow and ice management (http://stormwater.pca.state.mn.us/index.php/Cold_climate_impact_on_runoff_man agement#Snow_and_ice_management).

Materials specifications

Specifications for infiltration basin, infiltration trench, dry well, and underground infiltration practices are provided below. The table below shows a comparison of different material specifications for infiltration practices. To view this information in an Excel file, link here (http://stormwater.pca.state.mn.us/index.php/File:Materials_specification s_infiltration_practices.xlsx).

Observation well

Infiltration practices should include an **observation well**, consisting of an anchored 4- to 6-inch diameter perforated PVC pipe fitted with a lockable cap installed flush with or above the ground surface. See MNDOT Manual Section 3248 (http://www.dot.state.mn.us/pre-letting/spec/2016/2016specbook.pdf) for design guidance on PVC pipes. It is recommended to install one well per 50 feet of length in the infiltration practice. For **permeable pavement (https://stor mwater.pca.state.mn.us/index.php?title=Permeable_pavement)** systems with greater than 10,000 square feet of **contributing drainage area (https://stormwater.pca.state.mn.us/index.php?title=Contributing_drainage_area_to_ stormwater BMPs)**, the observation well should be placed at the downstream end of the practice.

The purpose of the observation well is to monitor water drainage in the infiltration practice. Check observation wells within the specified **drawdown time** (24 or 48 hours) after a storm event. Standing water in the well is an indication of clogging of the infiltration system. Observation wells should not be used for water quality monitoring since monitoring wells extend into underlying groundwater.

A detail of a typical infiltration cross-section, which includes an observation well, can be found at this link (http://storm water.pca.state.mn.us/index.php/File:INFILTRATION_TRENCH_INFILTRATION_TRENCH_SECTION_%281%29.p df). This detail includes construction sequencing and other design information.

Emergency overflow structure

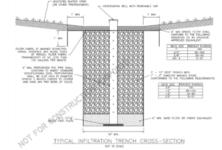
Infiltration trench and infiltration basin

- All infiltration beds should be designed with overflow spillway for extreme storm events
- Properly designed vegetated filters are required from the overflow of all infiltration systems

Dry well

• A surcharge pipe should be constructed as an emergency overflow device for dry wells which receive runoff directly from a roof leader

High flow bypass structure



Schematic of a typical infiltration trench cross-section, including an observation well.

Infiltration trench and infiltration basin

- A high flow bypass or diversion structure should be included to safely convey high flows from large storm events
- Control in beds is usually provided as an outlet control structure
- Modified inlet box with internal concrete weir (or weir plate) and low-flow orifice is common
- High flows must be bypassed within 48 hours and allow the infiltration practice to meet the 48 hour drawdown requirement

Underground infiltration

- A flow bypass structure should be located upgradient of the infiltration structure to convey high flows around the structure
- High flows must be bypassed within 48 hours and allow the infiltration practice to meet the 48 hour drawdown requirement

Buffer vegetation

Infiltration basin, underground infiltration, dry well

- Keep adjacent vegetation from forming overhead canopy above infiltration practices. This keeps leaf litter, fruits, and other vegetative material from clogging stone
- For infiltration basins, trees may be planted along side slopes but must be 15 ft away from pipes and 25 ft away from risers.

Infiltration trench

- Grass strip or other vegetated buffer at least 20 feet wide should be maintained around trenches accepting surface runoff as sheet flow
- Slope of filter strip should be approximately 1% along entire length and 0% across width

Surface cover

Infiltration basin

• For stone cover: apply a two inch layer of pea gravel or river stone

Infiltration trench

• 3-inch layer of river stone or pea gravel with filter fabric and additional aggregate on top

Underground infiltration

- Permeable topsoil and planted with grass
- Deep-rooted plants, such as prairie grass, should be used to increase infiltration capacity
- Bottom and side slopes of basin should be planted with dense water-tolerant grass (such as fescues)
- Soil cover (12 18 inches) should be maintained above infiltration bed to allow for a healthy vegetative cover
- Overly infiltration bed with native grasses, meadow mix, or other low-growing, dense vegetation
- In place of turf, use basin liner of 6 to 12 inches of fill material (coarse sand)

Dry well

Covered by a minimum of 12 inches of topsoil

Filter bed (stone)

Infiltration basin

- I to 3-foot thick layer of double-washed, uniform (uniformity coefficient of 2 or smaller with a void ratio of 0.40) aggregate between 1.5 and 3.0 inch diameter
- MNDOT Coarse Aggregate Bedding (Section 3149 G.2) (http://www.dot.state.mn.us/pre-letting/spec/2000/sp3101.pdf) ranging 1-2 inch in gradation

Infiltration trench

- 1 to 4-foot thick layer of double-washed, uniform (uniformity coefficient of 2 or smaller with a void ratio of 0.40) aggregate between 1.5 and 3.0 inch diameter
- MNDOT Coarse Aggregate Bedding (Section 3149 G.2) ranging 1-2 inch in gradation

Underground infiltration

- 1 to 4-foot thick layer of double-washed, uniform (uniformity coefficient of 2 or smaller with a void ratio of 0.40) aggregate between 1.5 and 3.0 inch diameter
- MNDOT Coarse Aggregate Bedding (Section 3149 G.2) ranging 1-2 inch in gradation

Dry well

- Double-washed, uniform (uniformity coefficient of 2 or smaller with a void ratio of 0.40) aggregate between 1.5 and 3.0 inch diameter
- MNDOT Coarse Aggregate Bedding (Section 3149 G.2) ranging 1-2 inch in gradation

Filter fabric

Infiltration basin

- Top of geotextile should be 6-12 inches below upper surface of drainage media with 12 inch overlap of fabric seams
- Fabric should be placed between side walls and between stone and gravel layers but not on the basin bottom
- Geotextile must meet MNDOT Type I Non-woven geotextile (http://dot.state.mn.us/mnroad/nrra/structure-teams/geotechnical/files/specifications/mndot-guideli nes-geotextiles-for-soil-field-personnel-pavements-draft-6-12-2020.pdf)
- Examples: Mirafi 140N, Amoco 4547, and Geotex 451

Infiltration trench

Design criteria for infiltration - Minnesota Stormwater Manual

- Surround fill on sides with filter fabric with 12 inch overlap of fabric seams. A separate piece should be used for the top layer to act as a failure plane
- Fabric should be placed between side walls and between stone and gravel layers but not on the trench bottom
- Geotextile must meet MNDOT Type I non-woven geotextile
 Every discrete Minor 140NLA wave 4547
- Examples: Mirafi 140N, Amoco 4547, and Geotex 451

Underground infiltration

- Surround fill on sides with filter fabric with 12 inch overlap of fabric seams
- Geotextile must meet MNDOT Type I non-woven geotextile
- Examples: Mirafi 140N, Amoco 4547, and Geotex 451

Dry well

- Geotextile on trench sides and top keyed into drywell
- Geotextile must meet MNDOT Type I non-woven geotextile
- Examples: Mirafi 140N, Amoco 4547, and Geotex 451

Bottom

Infiltration basin, infiltration trench, underground infiltration, dry well Any of the following can be used on the bottom:

- 6 to 8 inch clean sand (1-2 percent fines or less)
- 3/8 inch pea gravel
- 6-12 inch layer of MNDOT Fine Filter Aggregate (Section 3149 J.2)
- 12 inch of gravel or sand (good for cold climates)

In-situ soils

Infiltration basin, infiltration trench, underground infiltration, dry well

NRCS Type A and B soils are the most efficient soils for proper infiltration. Type C soils are acceptable of the infiltration practice meets the required 48 hour drawdown period

Miscellaneous considerations

- All material must be placed such that compaction is avoided. See the Construction Specifications (http://stormwater.pca.state.mn.us/index.php/Construction_specifications_for_infiltration) webpage for more details
- · For infiltration trench and underground infiltration, plastic, aluminum, or concrete gallery frames can be inserted to increase the runoff capture storage volume.

Infiltration material specifications

Link to this table. To open this table in Excel format, link here (http://stormwater.pca.state.mn.us/index.php/File:Materials_specifications_all_infiltration.xlsx).

Component	Infiltration basin	Bioinfiltration basin	Infiltration trench	Underground infiltration	Dry well	Dry swale with check dams	Permeable pavement	tren
Observation well	e.mn.us/pre-letti Install one per 5	nch Schedule 40 PVC pe ng/spec/2016/2016specb 0 ft. of length of infiltrati ush with or above the su	ook.pdf) with a loc on practice	kable cap and anchor			 Vertical 4- to 6-inch Schedule 4 PVC perforated pipe according t MNDOT Manux Section 3248 (h p://www.dot.stat e.mn.us/pre-letti ng/spec/2016/2C 16specbook.pdf with a lockable cap and anchor plate within gravel portion o bed Install one per 5 ft. of length of infiltration practice Install at downstream end of all permeable pavement systems >10,000 ft² contributing drainage area 	o al tt t i)) f 0 -

6/6/22, 2:55 PN	1		Design criteria for	r infiltration - Minr	nesota Stormwate	er Manual		
Component	Infiltration basin	Bioinfiltration basin	Infiltration trench	Underground infiltration	Dry well	Dry swale with check dams	Permeable pavement	tren
Emergency overflow structure	 spillway for extr Properly designed overflow of all in See detail (http://www.all.extraction.com/all	eds should be designed w reme storm events ed vegetated filters are rec nfiltration systems //stormwater.pca.state.mn /FILTRATION_BASIN_I odf)	quired from the nus/images/0/01/Infile	NA	A surcharge pipe should be constructed as an emergency overflow device for Dry Wells which receive runoff directly from a roof leader. [6] (http:// www.bfenvironm ental.com/pdfs/Fr enchdrains.pdf)		See Design criteria for permeable pavement	Tree be d eme pipe
High flow bypass structure	 to safely convey Control in beds i structure Modified inlet b and low-flow or High flows must 	bass or diversion structure high flows from large sto is usually provided as an ox with internal concrete ifice is common t be bypassed within 48 h ice to meet the 48 hour d	orm events outlet control weir (or weir plate) ours and allow the	 A flow bypass structure should be located upgradient of the infiltration structure to convey high flows around the structure High flows must be bypassed within 48 hours and allow the infiltration practice to meet the 48 hour 		NA	NA	NA
Buffer vegetation	 an overhead can practices. This k and other vegeta clogging stone. Trees may be pla 	egetation from forming opy above infiltration reeps leaf litter, fruits, tive material from anted along side slopes t away from pipes and risers	 Grass strip or other vegetated buffer at least 20 feet wide should be maintained around trenches accepting surface runoff as sheet flow Slope of filter strip should be approximately 1% along entire length and 0% across width 	litter, fruits, and o material from clog	ead canopy above es. This keeps leaf ther vegetative	 Planted with turf grass, tal meadow grasses, decorative herbaceous cover, or trees See MNDOT Standards and Specs Section 3876 for Seed Requirement (http://www. dot.state.mn. us/pre-lettin g/spec/2016/ 2016specboo k.pdf) for Dry Swales If receiving runoff from an impervious area, construct an adjacent grass filter strip, gravel flow spreade to provide pretreatment 	r	

For stone cover: apply a two inch layer of pea gravel or river stone	 consist of aged, double-shredded hardwood bark mulch or shredded leaf mulch Sand, if used, must comply with MNDOT Sand Cover in Section 3149 K (http://www.dot.s tate.mn.us/pre-let 	stone or pea gravel with filter fabric and additional aggregate on top	 as prairie grass, should be used to increase infiltation capacity Bottom and side slopes of basin should be planted with dense water- tolerant grass (such as fescues) Soil cover (12 - 18 inches) should be maintained above infiltration bed to allow for a healthy vegetative cover Overly infiltration bed with native grasses, meadow mix, or other low- growing, dense vegetation In place of turf, use basin liner of 6 to 12 inches of 	Covered by a minimum of 12 inches of topsoil	survival See MNDOT Standards and Specs Section 3882 (http://www. dot.state.mn. us/pre-lettin g/spec/2016/ 2016specboo k.pdf) for Mulch Requirements Erosion control matting can be used in	 to 25% void content¹ Plastic Grid Pavers: depends on fill material Concrete Grid Pavers: 3.5 inch thick layer with 20% to 50% void content Concrete Interlocking Pavers: 3 inch thick layer with 5% to 15% void content¹ Mit Thislances 	•
NA	NA	NA	NA	NA	NA	Choker Layer: A 2 to 8 inch bedding coarse of Medium Filter Aggregate (MNDOT Section 3149 J.1 (htt p://www.dot.state.mn.u s/pre-letting/spec/2016/ 2016specbook.pdf)) is usually placed over the	NA
	a two inch layer of pea gravel or river stone	 For stone cover: apply a two inch layer of pea gravel or river stone For stone cover: apply a two inch layer of pea gravel or river stone Mulch (2-3 inch layer) must be free of weeds and consist of aged, double-shredded hardwood bark mulch or shredded leaf mulch Sand, if used, must comply with MNDOT Sand Cover in Section 3149 K (http://www.dot.s tate.mn.us/pre-let ting/spec/2016/20 16specbook.pdf) Deep-rooted, native species for vegetative cover If sod is used, grow over permeable soils 	 vegetation on basin side slopes and floor: Dry- and mesic- prairie species for side slopes Wet meadow species for lower portions Mulch (2-3 inch layer) must be free of weeds and consist of aged, double-shredded hardwood bark mulch or shredded leaf mulch Sand, if used, must comply with MNDOT Sand Cover in Section 3149 K (http://www.dot.s tate.mu.s/pre-let ting/spec/2016/20 Iospecbook.pdf) Deep-rooted, native species for vegetative cover If sod is used, grow over permeable soils 	 For stone cover: apply at wo inch layer of prairie species for side slopes in side slopes in side slopes in the species for species for side slopes in the species for specie	 For stone cover: apply a two inch layer or river stone agraved or river stone agraved or river stone solution of parts of the species for side slopes and floor: Mulch (2-3 inch layer of river portions have on the layer of river stone shredded lead mulch or shredded lead mulch or shredded lead mulch for Section 3149 K (http://www.dot.st tate.mu.s/pre-let ting/species for solution agragation of the species for solution and side slopes of basin side slopes of basin side slopes or basin should be mean ow species for solution and side slopes of basin agras (sauch as focues) as focues of the should be free of weeds and should be free of mulch Sand, if used, must comply with MNDOT system of river stone over in Section 3149 K (http://www.dot.st tate.mu.s/pre-let ting/spec/2016/20 native species for weeds and ove infiltration tates or topsoil ad mulch or should be minimum of 12 maintained inches of topsoil ad should be minimum of 12 maintained inches of topsoil ad the species for mulch. Sand, if used, must comply with MNDOT system of the flow- graved or river store is store over section and side slopes in the species for mulch. Deep-roted, mative species for mulch is used, grow over permeable soils 	For stone cover: apply a two inch layer of roots of system consist of aged, gravel or river stone	 For store cover apply a two inch layer of river store of regenerative cover apply with MDOT store of regenerative cover apply and internal additional aggregate and rive species for regenerative cover apply with MDOT store of regenerative cover apply with MDOT store of regenerative cover apply with MDOT store of regenerative cover apply and in the data store of regenerative cover apply with MDOT store of regenerative cover apply with MDOT store of regenerative cover apply and in the data store of regenerative cover apply and its store of the regenerative cover apply with MDOT store of regenerative cover apply with MDOT store of regenerative cover apply and its store of the regenerative cover apply with MDOT store of regenerative cover apply and its store of the regenerative cover apply and its store of the regenerative cover apply with MDOT store of regenerative cover apply and its store of the regenerative cover apply and the regenerative c

Component	Infiltration basin	Bioinfiltration basin	Infiltration trench	Underground infiltration	Dry well	Dry swale with check dams	Permeable pavement	tren
Filter bed (stone)	 1 to 3-foot thick layer of double- washed, uniform (uniformity coefficient of 2 or smaller with a void ratio of 0.40) aggregate between 1.5 and 3.0 inch diameter MNDOT Coarse Aggregate Bedding (Section 3149 G.2) (http://ww w.dot.state.mn.u s/pre-letting/spe c/2016/2016spe cbook.pdf) ranging 1-2 inch in gradation 	Bioinfiltration engineered soil mix	 washed, unit coefficient of void ratio of between 1.5 diameter MNDOT Co. Bedding (Se p://www.dot ng/spec/201 	hick layer of double- form (uniformity of 2 or smaller with a '0.40) aggregate and 3.0 inch parse Aggregate action 3149 G.2) (htt astate.mn.us/pre-letti 6/2016specbook.pdf) inch in gradation	 Double- washed, uniform (uniformity coefficient of 2 or smaller with a void ratio of 0.40) aggregate between 1.5 and 3.0 inch diameter MNDOT Coarse Aggregate Bedding (Section 3149 G.2) (http://ww w.dot.state. mn.us/pre- etting/spec. 2016/2016 speebook.p df) ranging 1-2 inch in gradation 	20-30 inch layer of permeable manufactured soil mixture meeting the bioinfiltration soil mix	 Reservoir Layer: 1 to 4-foot deep bed of double- washed, uniform (uniformity coefficient of 2 or smaller with a void ratio of 0.40) aggregate between 1.5 and 3.0 inch diameter MNDOT Coarse Aggregate Bedding (Sectior 3149 G.2) (htt p://www.dot.stat e.mn.us/pre-letti ng/spec/2016/20 16specbook.pdf) ranging 1-2 inch in gradation 	r 3' of r Soil n
Filter fabric	 Top of geotextile should be 6-12 inches below upper surface of drainage media with 12" overlap of fabric seams Fabric should be placed between stone and gravel layers but not on the basin bottom Geotextile must meet MNDOT Type I Non-woven geotextile (https://www.usfabricsinc.com/specifications/dot/minnesota) Examples: Mirafi 140N, Amoco 4547, and Geotex 451. 	 Geotextile or a pea gravel filter separating stone and soil media Geotextile must meet MNDOT Type I non-woven geotextile (https://www.usfa bricsinc.com/spe cifications/dot/mi nnesota) Examples: Mirafi 140N, Amoco 4547, and Geotex 451 	 Surround fil on sides with filter fabric with 12" overlap of fabric seams A separate piece should be used for the top layer to act as a failure plane Fabric should be placed between side walls and between stor walls and between stor walls and between stor walls and between stor and gravel layers but no on the trench bottom. Geotextile MNDOT Type I non- woven geotextile (h ps://www.us bricsinc.com pecifications dot/minneso a) Examples: Mirafi 140N Amoco 454' and Geotex 451 	h S. Surround fill on sides with filter fabric with 12" overlap of fabric d seams Geotextile must meet MNDOT type I non- woven ot geotextile n (https://ww w.usfabricsi nc.com/spe cifications/ dot/minnes ota) Examples: Mirafi fa 140N, n/s Amoco s/ 4547, and t Geotex 451	 Geotextile on trench sides and top keyed into drywell Geotextile must meet MNDOT Type I non-woven geotextile (https://ww w.usfabrics inc.com/sp ecification s/dot/minn esota) Examples: Mirafi 140N, Amoco 4547, and Geotex 451 	gravel layer, it must meet MNDOT Type I non- woven	 Filter Blanket could include any of the following: 8 inch layer of sand or filter fabric (Fine Aggregate) 3" thick of 3/8" pea gravel MNDOT Type I non-wover geotextile (https://w ww.usfabri csinc.com/ specificati ons/dot/mi nnesota) Examples: Mirafi 140N, Amoco 4547, and Geotex 451 (K) Geotextile should be placed on filter bed sides between filter media and in-situ soils 	n i í

Component	Infiltration basin	Bioinfiltration basin	Infiltration trench	Underground infiltration	Dry well	Dry swale with check dams	Permeable pavement	tren
Bottom	 Any of the following can be used on the bottom: 6 to 8 inch clean sand (1-2% fines or less) 3/8" pea gravel 6-12" layer of MNDOT Fine Filter Aggregate (Section 3149 J.2) (http://ww w.dot.stat e.mn.us/p re-letting/ spec/201 6/2016spe cbook.pd f) 12" of gravel or sand (good for cold climates) 	 Any of the following can be used on the bottom: Uniformly graded, crushed, clean-washed stone. (less than 0.5% wash loss, by mass, per AASHTO T-11 wash loss test Grass turf that can be inundated for +72 hrs MNDOT Coarse Aggregate Bedding (Section 3149 G.2) (http://ww w.dot.state.mn.us/pre-l etting/spec/2016/2016s pecbook.pd f) Coarse organic material (composted mulch or erosion control mix) tilled into soil, soaked and allowed to dry 	 6 to 8 3/8" I 6-12" Aggrut.state book. 	following can be used inch clean sand (1-2% ea gravel layer of MNDOT Fin egate (Section 3149 J.2 mn.us/pre-letting/spe pdf) f gravel or sand (good	% fines or less) e Filter 2) (http://www.do c/2016/2016spec	and 3.0 inch diameter MNDOT Coarse Aggregate Bedding	Bottom Layer: 4" min depth of .75" crushed stone MNDOT Coarse Aggregate Bedding (Section 3149 G.2) (ht p://www.dot.state.mn.u s/pre-letting/spec/2016 2016specbook.pdf)	wasl t 0.75 1 dian

in-situ soils NRCS Type A and B soils are the most efficient soils for proper infiltration. Type C soils are acceptable of the infiltration practice meets the required 48 hou period

,			5					
Component	Infiltration basin	Bioinfiltration basin	Infiltration trench	Underground infiltration	Dry well	Dry swale with check dams	Permeable pavement	tren
Miscellaneous	All material must be p compaction is avoided Specifications webpag	. See the Construction	volume of trenche or concrete gallery inserted All mater	ial must be placed ion is avoided. See Specifications		 Non-erosive material such as wood, gabions, riprap, or concrete Underlay with geotextile and include 1/2 inch weep holes Construct check dams 6 to 12 inches high 9" min diameter rock on outside edges, 12-18" diameter larger rocks at core. Gaps filled with 4-6" diameter smaller rock Designed so pond behind dam will drawdown within 12 hours 		

¹Thickness will vary depending on traffic conditions. Typically, thicker configurations are needed for heavier traffic loads.

Materials specifications - filter media

Infiltration practices do not typically use engineered filter media. If **engineered media** (https://stormwater.pca.state.mn.us/index.php?title=Design_criteria_for_bioretention# Materials_specifications_-_filter_media) is used, all media should meet the specifications listed in the design criteria for bioretention basin webpage. Links are provided below.

- Filter media depth (http://stormwater.pca.state.mn.us/index.php/Design criteria for bioretention#Filter media depth)
- Performance specifications (http://stormwater.pca.state.mn.us/index.php/Design_criteria for bioretention#Performance specifications)
- Guidance for bioretention media composition (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_bioretention#Guidance_for_bioretention_media_comp osition)
 - Mix A (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_bioretention#Mix_A:_Water_quality_blend)
 - Mix B (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_bioretention#Mix_B:_Enhanced_filtration_blend)
 - Mix C (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_bioretention#Mix_C:_North_Carolina_State_University_water_quality_blend)
 - Mix D (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_bioretention#Mix_D)
 - Mix E (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_bioretention#Mix_E: MnDOT_3877.2_Type_G_27Filter_Topsoil_Borrow.27)
 - Mix F (http://stormwater.pca.state.mn.us/index.php/Design_criteria_for_bioretention#Mix_F:_Custom_Infiltration_Basin_Planting_Soil)
 - Other media (http://stormwater.pca.state.mn.us/index.php/Design criteria for bioretention#Other media)

Design procedure - design steps

The following steps outline a recommended design procedure for infiltration practices in compliance with the MPCA Construction Stormwater General Permit (https://storm water.pca.state.mn.us/index.php?title=Construction_stormwater_program) for new construction. Design recommendations beyond those specifically required by the permit are also included and marked accordingly.

When riser pipe outlets are used in infiltration basins, it is HIGHLY RECOMMENDED that they be constructed with manholes that either have locks or are sufficiently heavy to prevent easy removal.

Fencing of dry wells and infiltration trenches is neither necessary nor desirable. Infiltration basins may warrant fencing in some situations.

Step 1. Make a preliminary judgment

Make a preliminary judgment as to whether site conditions are appropriate for the use of an infiltration practice, and identify the function of the practice in the overall treatment system.

A. Consider basic issues for initial suitability screening, including:

- site drainage area (See the summary of infiltration practices for given drainage areas in the table below);
- site topography and slopes;
- soil infiltration capacity;
- regional or local depth to groundwater and bedrock;

https://stormwater.pca.state.mn.us/index.php?title=Design_criteria_for_infiltration#Step_10._Prepare_Vegetation_and_Landscaping_Plan

- site location/ minimum setbacks; and
 presence of active karst (https://stormwater.pca.state.mn.us/index.php?title=Karst).
- B. Determine how the infiltration practice will fit into the overall stormwater treatment system.
 - Decide whether the infiltration practice is the only bmp to be employed, or if are there other BMPs addressing some of the treatment requirements.
 - Decide where on the site the infiltration practice is most likely to be located.

Stormwater infiltration BMPs - contributing drainage area Link to this table

Stormwater BMP	Recommended contributing area	Notes
Infiltration Basin	50 acres or less	A natural or constructed impoundment that captures, temporarily stores and infiltrates the design volume of water into the surrounding naturally permeable soil over several days. In the case of a constructed basin, the impoundment is created by excavation or embankment.
Bioinfiltration Basin	5 acres or less	Bioinfiltration basins must meet the required 48 hour drawdown time and must be sized in order to allow for adequate maintenance. It is HIGHLY RECOMMENDED that bioinfiltration basins be designed to prevent high levels of bounce as submerging vegetation may inhibit plant growth. A maximum wet storage depth of 1.5 feet is HIGHLY RECOMMENDED.
Infiltration Trench	5 acres or less	
Dry Well Synonym: Infiltration Tube, French Drain, Soak- Away Pits, Soak Holes	1 acre or less (rooftop only)	
Underground Infiltration	10 acres or less	Though feasible, larger underground infiltration systems may cause groundwater contamination as water is not able to infiltrate through a surface cover. In addition, wind flocculation, UV degradation, and bacterial degradation, which provide additional treatment in surface systems, do not occur in underground systems. Because performance research is lacking for larger features, it is HIGHLY RECOMMENDED that the contributing drainage area to a single device not exceed 10 acres.
Dry Swale with Check Dams	5 acres or less	
Permeable Pavement	It is RECOMMENDED that external contributing drainage area not exceed the surface area of the permeable pavement. It is HIGHLY RECOMMENDED that external contributing drainage area not exceed twice the surface area of the permeable pavement	It is RECOMMENDED that external drainage area be as close to 100% impervious as possible. Field experience has shown that drainage area (pervious or impervious) can contribute particulates to the permeable pavement and lead to clogging. Therefore, sediment source control and/or pretreatment should be used to control sediment run-on to the permeable pavement section.
Tree Trench/Tree Box	up to 0.25 acres per tree	
ormwater-bmp-manual), W	est Virginia (http://www.dep.wv.gov/WWE/Programs/stormwater/MS4/Pages/Stormw	8_INFILTRATION_v1-9_03012011.pdf), North Carolina (https://deq.ne.gov/about/divisions/energy-mineral-land-resources/energy-mineral-land-permit-guidance/st vaterManagemenDesignandGuidanceManual.aspx), Maine (http://www.maine.gov/dep/land/stormwater/stormwa

ndbook.aspx), Connecticut (http://www.et.gov/deep/cwp/view.asp?a=2721&q=325704), Massachusetts (http://www.mass.gov/eea/agencies/massdep/water/regulations/massachusetts-stormwater-handbook.html), New York (http://www.dec.ny.gov/chemical/29072.html), Wisconsin (https://dnr.wi.gov/topic/Stormwater/standards/postconst_standards.html), Vermont (http://www.dec.ny.gov/chemical/29072.html), Output: 200-001/wd-08-200-pdf), Ontario (https://www.ontario.ca/document/stormwater-management-planning-and-design-manual), Pennsylvania (http://www.des.nat.gov/sites/dev/files/wbeh7iew/collection-8305)

Step 2. Confirm design criteria and applicability

Determine whether the infiltration practice must comply with the MPCA Construction Stormwater General (CSW) Permit (https://stormwater.pca.state.mn.us/index.php?title =Construction_stormwater_program). Check with local officials, Watershed management Organizations (WMOs), and other agencies to determine if there are any additional restrictions and/or surface water or watershed requirements that may apply.

Warning: If the infiltration practice must comply with the CSW permit, the following prohibitions apply:

- areas that receive discharges from vehicle fueling and maintenance;
- areas with less than three (3) feet of separation distance from the bottom of the infiltration system to the elevation of the seasonally saturated soils or the top of bedrock;
- areas that receive discharges from industrial facilities which are not authorized to infiltrate industrial stormwater under an NPDES/SDS Industrial Stormwater Permit
 issued by the MPCA;
- areas where high levels of contaminants in soil or groundwater will be mobilized by the infiltrating stormwater;
- areas of predominately Hydrologic Soil Group D (clay) soils;
- areas within 1,000 feet up-gradient, or 100 feet down-gradient of active karst features;
- areas within a Drinking Water Supply Management Area (DWSMA) as defined in Minn. R. 4720.5100, subp. 13., if the system will be located:
 - in an Emergency Response Area (ERA) within a DWSMA classified as having high or very high vulnerability as defined by the Minnesota Department of Health; or
 - in an ERA within a DWSMA classified as moderate vulnerability unless a regulated MS4 Permittee performed or approved a higher level of engineering review sufficient to provide a functioning treatment system and to prevent adverse impacts to groundwater; or
 - outside of an ERA within a DWSMA classified as having high or very high vulnerability, unless a regulated MS4 Permittee performed or approved a higher level of engineering review sufficient to provide a functioning treatment system and to prevent adverse impacts to groundwater; and
- areas where soil infiltration rates are more than 8.3 inches per hour unless soils are amended to slow the infiltration rate below 8.3 inches per hour.

Step 3. Perform field verification of site suitability

Warning: The Construction Stormwater permit includes the following requirements.

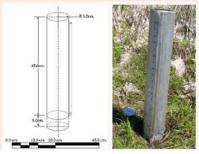
16.10. Permittees must provide at least one soil boring, test pit or infiltrometer test in the location of the infiltration practice for determining infiltration rates.

16.11. For design purposes, permittees must divide field measured infiltration rates by 2 as a safety factor or permittees can use soil-boring results with the infiltration rate chart in the Minnesota Stormwater Manual to determine design infiltration rates. When soil borings indicate type A soils, permittees should perform field measurements to verify the rate is not above 8.3 inches per hour. This permit prohibits infiltration if the field measured infiltration rate is above 8.3 inches per hour.

Designers should evaluate soil properties during preliminary site layout with the intent of installing infiltration practices on soils with the highest **infiltration rates** (**hydrologic soil group (https://stormwater.pca.state.mn.us/index.php?titl e=Design_infiltration_rates)** A and B). Preliminary planning for the location of an infiltration device may be completed using a county soil survey or the NRCS Web Soil Survey (http://websoilsurvey.nrcs.usda.gov/app/HomePage. htm). These publications provide HSG information for soils across Minnesota. To ensure long-term performance, however, field soil measurements (https://stormwater.pca.state.mn.us/index.php?title=Determining_soil_infiltration_rate s) are desired to provide site-specific data.

If the initial evaluation indicates that an infiltration practice would be a good BMP for the site, it is *RECOMMENDED* that soil borings (https://stormwater.pca.state.mn.us/index.php?title=Understanding_and_interpreting_soils_and_soil_boring_reports_for_infiltration_BMPs) or pits be dug within the proposed boundary of the infiltration practice to verify soil types and infiltration capacity characteristics and to determine the depth to groundwater and bedrock. Soil borings for building structural analysis are not acceptable. In all design scenarios, a minimum of one soil boring (two are

recommended) shall be completed to a depth 5 feet below the bottom of the proposed infiltration Stormwater Control



The modified Philip-Dunne infiltrometer, as well as other direct infiltration measurements such as double ring infiltrometers, are preferred methods for measuring infiltration rates, compared to soil borings and pits. Image: University of Minnesota, St. Anthony Falls Laboratory (http://stormwaterbook.safl. umn.edu/water-budget-measurement/infiltrati on).

Measure (SCM or BMP) (Dakota County Soil and Water Conservation District, 2012) per ASTM D1586 (ASTM (http://www.astm.org/), 2011). For infiltration SCMs with surface area between 1000 and 5000 square feet, two borings shall be made. Between 5000 and 10000 square feet, three borings are needed, and for systems with greater than 10000 square feet in surface area, 4 or more borings are needed. For each additional 2500 square feet beyond 12,500 square feet, an additional soil boring should be made. Soil borings must be undertaken during the design phase (i.e. prior to the commencement of construction) to determine how extensive the soil testing will be during construction. Borings should be completed using continuous split spoon sampling, with blow counts being recorded to determine the level of compaction of the soil. Soil borings are needed to understand soil types, seasonally high groundwater table elevation, depth to karst, and bedrock elevations.

Recommended number of soil borings, pits or permeameter tests for bioretention design. Designers select one of these methods. Link to this table

Surface area of stormwater control measure (BMP)(ft ²)	Borings	Pits	Permeameter tests
< 1000	1	1	5
1000 to 5000	2	2	10
5000 to 10000	3	3	15
>10000	4 ¹	4^{1}	20^{2}

¹an additional soil boring or pit should be completed for each additional 2,500 ft² above 12,500 ft²

²an additional five permeameter tests should be completed for each additional 5,000 ft² above 15,000 ft²

It is *HIGHLY RECOMMENDED* that soil profile descriptions be recorded and include the following information for each soil horizon or layer (Source: *Site Evaluation for Stormwater Infiltration* (https://dnr.wi.gov/topic/Stormwater/standards/postconst_standards.html), Wisconsin Department of Natural Resources Conservation Practice Standards 2004):

- thickness, in inches or decimal feet;
- Munsell soil color notation;
- soil mottle or redoximorphic feature color, abundance, size and contrast;
- USDA soil textural class with rock fragment modifiers;
- soil structure, grade size and shape;
- soil consistence, root abundance and size;
- soil boundary; and
- occurrence of saturated soil, impermeable layers/lenses, ground water, bedrock or disturbed soil.

It is RECOMMENDED that a standard soil boring form be used. A good example is File:Boring Pit Log form.docx. The NRCS Field Book for Describing and Sampling Soils (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/ref/?cid=nrcs142p2_054184) provide detailed information for identifying soil characteristics. Munsell color charts can be found here (https://munsell.com/color-products/color-communications-products/environmental-color-communication/munsell-soil-color-charts/).

Warning: A separation distance of 3 feet is REQUIRED between the bottom of the infiltration practice and the elevation of the seasonally high water table (saturated soil) or top of bedrock (i.e. there must be a minimum of 3 feet of undisturbed soil beneath the infiltration practice and the seasonally high water table or top of bedrock).

It is HIGHLY RECOMMENDED that the field verification be conducted by a qualified geotechnical professional.

Step 4. Compute runoff control volumes

Warning: If the infiltration practice is being designed to meet the requirements of the MPCA Permit, the *REQUIRED* treatment volume is the water quality volume (V_{wq}) of 1 inch of runoff from the new impervious surfaces created from the project. If part of the overall V_{wq} is to be treated by other BMPs, subtract that portion from the V_{wq} to determine the part of the V_{wq} to be treated by the infiltration practice.

The design techniques in this section are meant to maximize the volume of stormwater being infiltrated. If the site layout and underlying soil conditions permit, a portion of the Channel Protection Volume (V_{p10}) , overbank Flood Protection Volume (V_{p10}) , and the Extreme Flood Volume (V_{p100}) may also be managed in the infiltration practice.

Step 5. Determine infiltration type and size practice

Once the physical suitability evaluation is complete (Step 3), it is *HIGHLY RECOMMENDED* that the designer apply the better site design (https://stormwater.pca.state.mn. us/index.php?title=Better_site_design) principles in sizing and locating the infiltration practice(s) on the development site.

Select design variant

After following the steps outlined above, the designer will presumably know the location of naturally occurring permeable soils, the depth to the water table, bedrock or other impermeable layer, and the contributing drainage area. Given the steps performed in the physical suitability evaluation (Step 3), identify the most suitable location for the infiltration practice. Given the water quality volume and the drainage area, select the appropriate infiltration practice for the first iteration of the design process. See the section on BMPs for stormwater infiltration (http://stormwater.pca.state.mn.us/index.php/BMPs_for_stormwater_infiltration#Infiltration_trench) for more information.

Note: Information collected during the site suitability evaluation (see Steps 1 and 3) should be used to explore the potential for multiple infiltration practices versus relying on a single infiltration facility. The use of smaller infiltration practices dispersed around a development is usually more sustainable than a single regional facility that is more likely to have maintenance and groundwater **mounding (https://stormwater.pca.state.mn.us/index.php?title=Stormwater_infiltration_and_groundwater_mounding)** problems (Source: Stormwater post-construction technical standards, Wisconsin Department of Natural Resources Conservation Practice Standards (https://dnr.wi.gov/topic/Stormwater/standards/postconst_standards.html). See Using the treatment train approach to BMP selection (http://stormwater.pca.state.mn.us/index.php/Using_the_treatmen t_train_approach_to_BMP_selection) for more information on selecting multiple BMPs at a site.

Warning: Infiltration practices shall discharge through the soil or filter media in 48 hours or less. Additional flows that cannot be infiltrated or filtered in 48 hours should be routed to bypass the system through a stabilized discharge point.

Experience has demonstrated that, although the drawdown period is 48 hours, there is often some residual water pooled in the infiltration practice after 48 hours. This residual water may be associated with reduced head, water gathered in depressions within the practice, water trapped by vegetation, and so on. The drawdown period is therefore defined as the time from the high water level in the practice to 1 to 2 inches above the bottom of the facility. This criterion was established to provide the following: wet-dry cycling between rainfall events; unsuitable mosquito breeding habitat; suitable habitat for vegetation; aerobic conditions; and storage for back-to-back precipitation events. This time period has also been called the period of inundation.

Caution: It is *HIGHLY RECOMMENDED* that the drawdown time for infiltration practices is 24 hours or less from the peak water level in the practice when discharges are to a trout stream.

Determine site infiltration rates (for facilities with infiltration and/or recharge)

For design purposes, there are two ways of determining the soil infiltration rate. The first, and preferred method, is to field-test the soil infiltration rate using appropriate methods described below. The other method uses the typical infiltration rate of the most restrictive underlying soil (determined during soil borings).

If infiltration rate measurements are made, a minimum of one infiltration test in a soil pit must be completed at the elevation from which exfiltration would occur (i.e. interface of gravel drainage layer and in situ soil). When the SCM surface area is between 1000 and 5000 square feet, two soil pit measurements are needed. Between 5000 and 10000 square feet of surface area, a total of three soil pit infiltration measurements should be made. Each additional 5000 square feet of surface area triggers an additional soil pit.

Recommended number of soil borings, pits or permeameter tests for bioretention design. Designers select one of these methods. Link to this table

Surface area of stormwater control measure (BMP)(ft ²)	Borings	Pits	Permeameter tests
< 1000	1	1	5
1000 to 5000	2	2	10
5000 to 10000	3	3	15
>10000	4 ¹	4^{1}	20^{2}

 1 an additional soil boring or pit should be completed for each additional 2,500 ft 2 above 12,500 ft 2

²an additional five permeameter tests should be completed for each additional 5,000 ft² above 15,000 ft²

The median measured infiltration rate should be utilized for design. Soil pits should be dug during the design phase and should be a minimum of two feet in diameter for measurement of infiltration rate. Infiltration testing in the soil pit can be completed with a double-ring infiltrometer or by filling the pit with water and measuring stage versus time. If the infiltration rate in the first pit is greater than 2 inches per hour, no additional pits shall be needed.

Alternatively, a Modified Philip-Dunne permeameter can be used to field test infiltration rate. Modified Philip-Dunne permeameter tests may be made in conjunction with soil borings or may be completed using a handheld soil auger. Borings should be lined with a plastic sleeve to prevent infiltration from the sides of the borehole (i.e. restrict flow to vertical infiltration). Soil borings should be filled with water. The time for the borehole to drain should be recorded and divided by the initial ponding depth in the borehole to provide an infiltration rate measurement. The design infiltration rate should be the lower of the median soil pit infiltration rate or the median borehole method infiltration rate. For information on conducting soil borings see Understanding and interpreting soils and soil boring reports for infiltration BMPs (http://stormwater.pca.stat e.mn.us/index.php/Understanding_and_interpreting_soils_and_soil_boring_reports_for_infiltration_BMPs).

NOTE: In the table above, the recommended number of permeameter tests increases by 5 tests per each additional 5000 square feet of surface area. For larger sites, this can result in a very large number of samples. There may be situations where fewer permeameter tests may be used (5 is the minimum). For example, in situations where the variability in saturated hydraulic conductivity between measurements is not great, fewer samples may be taken. One method for determining the number of samples is to plot standard deviation versus number of samples. Measurements may be halted when the standard deviation becomes relatively constant from one sample to the next. In the example to the right the standard deviation flattens at about 7 to 10 samples. Therefore, 7 to 10 samples would be an appropriate number of samples for this situation.

For information on conducting soil infiltration rate measurements, see Determining soil infiltration rates.

Design criteria for infiltration - Minnesota Stormwater Manual

If the infiltration rate is not measured, use the table below to estimate an infiltration rate for the design of infiltration practices. These infiltration rates represent the long-term infiltration capacity of a practice and are not meant to exhibit the capacity of the soils in the natural state.

Caution: Select the design infiltration rate from the table based on the least permeable soil horizon within the first 5 feet below the bottom elevation of the proposed infiltration practice

Caution: The table for design infiltration rates has been modified. Field testing is recommended for gravelly soils (HSG A; GW and GP soils; gravel and sandy gravel soils). If field-measured soil infiltration rates exceed 8.3 inches per hour, the Construction Stormwater permit requires the soils be amended. Guidance on amending these soils can be found here.

Design infiltration rates, in inches per hour, for A, B, C, and D soil groups. Corresponding USDA soil classification and Unified soil Classifications are included. Note that A and B soils have two infiltration rates that are a function of soil texture.*

The values shown in this table are for uncompacted soils. This table (https://stormwater.pca.state.mn.us/index.ph p?title=General_relationship_of_soil_bulk_density_to_root_growth_based_on_soil_texture) can be used as a guide to determine if a soil is compacted. For information on alleviating compacted soils, link here (https://storm water.pca.state.mn.us/index.php/Alleviating_compaction_from_construction_activities). If a soil is compacted, reduce the soil infiltration rate by one level (e.g. for a compacted B(SM) use the infiltration rate for a B(MH) soil).

Link to this table

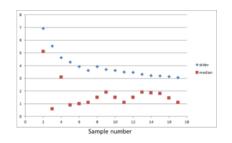


Illustration of how to determine the appropriate number of permeameter samples. When the standard deviation for all measurements flattens out with successive measurements, collection of additional permeameter tests may be halted, provided a minimum of 5 samples have been collected.

Hydrologic soil group	Infiltration rate (inches/hour)	Infiltration rate (centimeters/hour)	Soil textures	Corresponding Unified Soil Classification
Α	conduct field infiltration tests or amend soils. ^b See Guid	ters per hour) may be used, it is <i>Highly recommended</i> that you lance for amending soils with rapid or high infiltration rates and a state.mn.us/index.php?title=Determining_soil_infiltration_rates).	gravel sandy gravel	GW - well-graded gravels, sandy gravels GP - gap-graded or uniform gravels, sandy gravels
	1.63 ^a	4.14	silty gravels gravelly sands sand	GM - silty gravels, silty sandy gravels SW - well-graded gravelly sands SW - uniformly graded sands
	0.8	2.03	sand loamy sand sandy loam	SP - gap-graded or poorly graded sands
В	0.45	1.14		SM - silty sands, silty gravelly sands
	0.3	0.76	loam, silt loam	MH - micaceous silts, diatomaceous silts, volcanic ash
С	0.2	0.51	Sandy clay loam	ML - silts, very fine sands, silty or clayey fine sands
D	0.06	0.15	clay loam silty clay loam sandy clay silty clay clay	GC - clayey gravels, clayey sandy gravels SC - clayey sands, clayey gravelly sands CL - low plasticity clays, sandy or silty clays OL - organic silts and clays of low plasticity CH - highly plastic clays and sandy clays OH - organic silts and clays of high plasticity

Design criteria for infiltration - Minnesota Stormwater Manual

*NOTE that this table has been updated from Version 2.X of the Minnesota Stormwater Manual. The higher infiltration rate for B soils was decreased from 0.6 inches per hour to 0.45 inches per hour and a value of 0.06 is used for D soils (instead of < 0.2 in/hr).

Source: Thirty guidance manuals and many other stormwater references were reviewed to compile recommended infiltration rates. All of these sources use the following studies as the basis for their recommended infiltration rates: (1) Rawls, Brakensiek and Saxton (1982); (2) Rawls, Gimenez and Grossman (1998); (3) Bouwer and Rice (1984); and (4) Urban Hydrology for Small Watersheds (NRCS). SWWD, 2005, provides field documented data that supports the proposed infiltration rates. (view reference list)

^aThis rate is consistent with the infiltration rate provided for the lower end of the Hydrologic Soil Group A soils in the Stormwater post-construction technical standards, Wisconsin Department of Natural Resources Conservation Practice Standards (https://dnr.wi.gov/topic/Stor mwater/standards/postconst_standards.html).

^bThe infiltration rates in this table are recommended values for sizing stormwater Manual in 2005. Additional technical review resulted in an update to the table in 2011. Over the past 5 to 7 years, several government agencies revised or developed guidance for designing infiltration practices. Several states now require or strongly recommend field infiltration tests. Examples include North Carolina, New York, Georgia, and the City of Philadelphia. The states of Washington and Maine strongly recommend field testing for infiltration rates, but both states allow grain size analyses in the determination of infiltration rates. The Minnesota Stormwater Manual strongly recommends field testing for infiltration rate, but allows information from soil borings or pits to be used in determining infiltration rate. A literature review suggests the values in the design infiltration rate table (https://stormwater.pca.state.mn.us/index.php?title=Design_infiltration rate. A literature review suggests the values in the design infiltration rate table (https://stormwater.pca.state.mn.us/index.php?title=Design_infiltration rate.

tion_rates) are not appropriate for soils with very high infiltration rates. This includes gravels, sandy gravels, and uniformly graded sands. Infiltration rates for these geologic materials are higher than indicated in the table. References: Clapp, R. B., and George M. Hornberger. 1978. Empirical equations for some soil hydraulic properties. Water Resources Research. 14:4:601–604; Moynihan, K., and Vasconcelos, J. 2014. SWMM Modeling of a Rural Watershed in the Lower Coastal Plains of the United States (https://www.chijournal.org/Journals/PDF/C372). Journal of Water Management Modeling. C372; Rawls, W.J., D. Gimenez, and R. Grossman. 1998. Use of soil texture, bulk density and slope of the water retention curve to predict saturated hydraulic conductivity Transactions of the ASAE. VOL. 41(4): 983-988; Saxton, K.E., and W. J. Rawls. 2005. Soil Water Characteristic Estimates by Texture and Organic Matter for Hydrologic Solutions. Soil Science Society of America Journal. 70:5:1569-1578.

The infiltration capacity and existing hydrologic regime of natural basins are inherently different than constructed practices and may not meet MPCA Permit requirements for constructed practices. In the event that a natural depression is being proposed to be used as an infiltration system, the design engineer must demonstrate the following information:

- infiltration capacity of the system under existing conditions (inches per hour)
- existing drawdown time for the high water level (HWL) and a natural overflow elevation.

The design engineer should also demonstrate that operation of the natural depression under post-development conditions mimics the hydrology of the system under predevelopment conditions.

If the infiltration rates are measured, the tests shall be conducted at the proposed bottom elevation of the infiltration practice. If the infiltration rate is measured with a double-ring infiltrometer the requirements of ASTM D3385 (http://www.astm.org) (Standard test method for infiltration rate of soils in field using double-ring infiltrometer) should be used for the field test.

Warning: The measured infiltration rate shall be divided by a safety factor of 2.

The safety factor of 2 adjusts the measured infiltration rates for the occurrence of less permeable soil horizons below the surface and the potential variability in the subsurface soil horizons throughout the infiltration site. This safety factor also accounts for the long-term infiltration capacity of the stormwater management facility.

Size infiltration practice

To meet requirements of the Stormwater General Permit (https://stormwater.pca.state.mn.us/index.php?title=Construction_stormwater_program) (CSW permit), the surface area (A_s, in square feet) of an infiltration practice is given by

 $A_s = V_w/D_o$

Where:

Vw = the water treatment volume of the area contributing runoff to the practice; and

 D_0 = the storage depth of ponded water in the practice.

The water treatment volume is given by

 $V_w = 0.0833 A_c$

Where 0.0833 = one inch of infiltration, as required by the permit; and $A_c =$ the impervious surface area contributing to the practice.

The entire water quality treatment volume is assumed to be instantaneously ponded in the infiltration practice.

For a BMP with sloped sides, the surface area (A_s) of an infiltration practice is the average area of the BMP, given by

 $A_s = (A_o + A_M)/2$

Where A_o is the surface area at the overflow; and A_M is the surface area at the top of the infiltration practice

The water treatment volume must drain with 48 hours (24 hours is RECOMMENDED if discharges from the practice are to a trout stream). The ponding depth can therefore be calculated knowing the infiltration rate of the soils underlying the practice.

Given the assumed infiltration rate for the practice, determine the maximum depth using the following equation

 $D = I_R D D T_{calc}$

where

D = maximum depth of practice (inches);

 $I_R = infiltration rate (inches/hour); and$

 DDT_{calc} = maximum drawdown time (48 hours).

Design criteria for infiltration - Minnesota Stormwater Manual

Field-measured infiltration rates are preferred. If the infiltration rate has not been measured, use the table below to determine the infiltration rate of the underlying soils. Note the numbers in the table are intentionally conservative based on experience gained from Minnesota infiltration sites. Two example calculations are provided below.

Sizing example 1

Assume a 5 acre watershed is 20 percent impervious. Runoff from this watershed will be routed to an infiltration practice that has an underlying loam soil.

- The treatment volume = 5 acres * 0.20 * 43560 square feet per acre * 0.0833 inches = 3630 cubic feet
- The ponded depth = 48 hours * 0.30 inches per hour = 14.4 inches = 1.2 feet
- The surface area of the practice = 3025 square feet

The dimensions of the infiltration practice can be determined to accommodate this area. For example, a square practice will be 55 feet wide by 55 feet long.

Sizing example 2

Assume a 7 acre watershed is 15 percent impervious. Runoff from this watershed will be routed to an infiltration practice where the underlying soil has a field-measured infiltration rate of 2 inches per hour.

Note:

- The infiltration rate must be divided by a safety factor of 2 since a field-measure rate is being used. This gives an infiltration rate of 1 inch per hour.
- The ponded depth = 48 hours * 1 inch per hour = 48 inches = 4 feet.
- The treatment volume = 7 acres * 0.15 * 43560 square feet per acre * 0.0833 inches = 3811 cubic feet
- The surface area of the practice = 3811 ft³ / 4.0 ft = 953 square feet

The dimensions of the infiltration practice can be determined to accommodate this volume. For example, a square practice will be 30.9 feet wide by 30.9 feet long.

If the infiltration practice does not require meeting the Construction Stormwater General Permit, methods other than the instantaneous volume method may be used. For example, as an infiltration basin fills during a rain event, water infiltrates the media. The infiltration area could be sized as follows

 $A_s = V_{wq} / (D_o + (I_R * t))$

Where:

 I_{R} = infiltration rate of underlying soils (feet per day);and

t = time during which the infiltration basin continues to capture runoff.

The time during which runoff continues to be delivered to the BMP varies with each event. As an example, for a 1 hour event on a B (SM) soil with an infiltration rate of 0.45 inches per hour, 1 acre of contributing impervious area, and a 1.5 foot ponding depth, A_s is 2361 square feet, compared to 2420 square feet considering only an instantaneous volume, or a decrease of 2.4 percent in the size of the basin. On an A soil with and infiltration rate of 1.6 inches per hour, A_s is 2222 square feet, or a decrease of 8.2 percent in the needed size of the basin. The area of the basin can also be decreased by increasing the ponded depth.

Infiltration practices may also be sized using different treatment goals. For example, the performance goal for Minimal Impact Design Standards (MIDS) is 1.1 inches, compared to 1 inch for the CSW permit. The MIDS performance goal was also based on initial modeling (http://www.pca.state.mn.us/index.php/view-document.html?gid=1 5664) that included infiltration during the rain event.

Warning: 48 hours is the REQUIRED maximum tf for infiltration under the CGP

Caution: The table for design infiltration rates has been modified. Field testing is recommended for gravelly soils (HSG A; GW and GP soils; gravel and sandy gravel soils). If field-measured soil infiltration rates exceed 8.3 inches per hour, the Construction Stormwater permit requires the soils be amended. Guidance on amending these soils can be found here.

Design infiltration rates, in inches per hour, for A, B, C, and D soil groups. Corresponding USDA soil classification and Unified soil Classifications are included. Note that A and B soils have two infiltration rates that are a function of soil texture.*

The values shown in this table are for uncompacted soils. This table (https://stormwater.pca.state.mn.us/index.php?title=General_relationship_of_soil_bulk_densit y_to_root_growth_based_on_soil_texture) can be used as a guide to determine if a soil is compacted. For information on alleviating compacted soils, link here (http s://stormwater.pca.state.mn.us/index.php/Alleviating_compaction_from_construction_activities). If a soil is compacted, reduce the soil infiltration rate by one level (e.g. for a compacted B(SM) use the infiltration rate for a B(MH) soil). Link to this table

Hydrologic soil group	Infiltration rate (inches/hour)	Infiltration rate (centimeters/hour)	Soil textures	Corresponding Unified Soil Classification
Α	Although a value of 1.63 inches per hour (4.14 centimeters per conduct field infiltration tests or amend soils. ^b See Guidance fo Determining soil infiltration rates (https://stormwater.pca.state.n	r amending soils with rapid or high infiltration rates and	gravel sandy gravel	GW - well-graded gravels, sandy gravels GP - gap-graded or uniform gravels, sandy gravels
	1.63 ^a 4.14		silty gravels gravelly sands sand	GM - silty gravels, silty sandy gravels SW - well-graded gravelly sands SW - uniformly graded sands

Hydrologic soil group	Infiltration rate (inches/hour)	Infiltration rate (centimeters/hour)	Soil textures	Corresponding Unified Soil Classification
	0.8	2.03	sand loamy sand sandy loam	SP - gap-graded or l poorly graded sands
В	0.45	1.14		SM - silty sands, silty gravelly sands
	0.3	0.76	loam, silt loam	MH - micaceous silts, diatomaceous silts, volcanic ash
С	0.2	0.51	Sandy clay loam	ML - silts, very fine sands, silty or clayey fine sands
D	0.06	0.15	clay loam silty clay loam sandy clay silty clay clay	GC - clayey gravels, clayey sandy gravels SC - clayey sands, clayey gravelly sands CL - low plasticity clays, sandy or silty clays OL - organic silts and clays of low plasticity CH - highly plastic clays and sandy clays OH - organic silts and clays of high plasticity

*NOTE that this table has been updated from Version 2.X of the Minnesota Stormwater Manual. The higher infiltration rate for B soils was decreased from 0.6 inches per hour to 0.45 inches per hour and a value of 0.06 is used for D soils (instead of < 0.2 in/hr). Source: Thirty guidance manuals and many other stormwater references were reviewed to compile recommended infiltration rates. All of these sources use the following studies as the basis for their recommended infiltration rates: (1) Rawls, Brakensiek and Saxton (1982); (2) Rawls, Gimenez and Grossman (1998); (3) Bouwer and Rice (1984); and (4) Urban Hydrology for Small Watersheds (NRCS). SWWD, 2005, provides field documented data that supports the proposed infiltration rates. (view reference list)

^aThis rate is consistent with the infiltration rate provided for the lower end of the Hydrologic Soil Group A soils in the Stormwater post-construction technical standards, Wisconsin Department of Natural Resources Conservation Practice Standards (https://dnr.wi.gov/topic/Stor mwater/standards/postconst_standards/html).

^bThe infiltration rates in this table are recommended values for sizing stormwater practices based on information collected from soil borings or pits. A group of technical experts developed the table for the original Minnesota Stormwater Manual in 2005. Additional technical review resulted in an update to the table in 2011. Over the past 5 to 7 years, several government agencies revised or developed guidance for designing infiltration practices. Several states now require or strongly recommend field infiltration tests. Examples include North Carolina, New York, Georgia, and the City of Philadelphia. The states of Washington and Maine strongly recommend field testing for infiltration rates, but both states allow grain size analyses in the determination of infiltration rates. The Minnesota Stormwater Manual strongly recommends field testing for infiltration rate, but both states allow grain size analyses in the determination of infiltration rates. The Minnesota Stormwater Manual strongly recommends field testing for infiltration rate, but allows information from soil borings or pits to be used in determining infiltration rate. A literature review suggests the values in the design infiltration rate table (https://stormwater.pca.state.mn.us/index.php?title=Design_infiltration_rates) are not appropriate for soils with very high infiltration rates. This includes gravels, and uniformly graded sands. Infiltration rates for these geologic materials are higher than indicated in the table.

References: Clapp, R. B., and George M. Hornberger. 1978. Empirical equations for some soil hydraulic properties. Water Resources Research. 14:4:601–604; Moynihan, K., and Vasconcelos, J. 2014. SWMM Modeling of a Rural Watershed in the Lower Coastal Plains of the United States (https://www.chijournal.org/Journals/PDF/C372). Journal of Water Management Modeling. C372; Rawls, W.J., D. Gimenez, and R. Grossman. 1998. Use of soil texture, bulk density and slope of the water retention curve to predict saturated hydraulic conductivity Transactions of the ASAE. VOL. 41(4): 983-988; Saxton, K.E., and W. J. Rawls. 2005. Soil Water Characteristic Estimates by Texture and Organic Matter for Hydrologic Solutions. Soil Science Society of America Journal. 70:5:1569-1578.

Step 6. Size outlet structure and/or flow diversion structure, if needed

It is *HIGHLY RECOMMENDED* that the outlet for the infiltration practice shall safely convey stormwater using all of the following mechanisms (Stormwater postconstruction technical standards, Wisconsin Department of Natural Resources Conservation Practice Standard (https://dnr.wi.gov/topic/Stormwater/standards/postconst_stan dards.html)).

Drawdown valve: infiltration systems may be designed with a drawdown valve for the removal of standing water for maintenance and winter diversion. **Emergency spillway**:

Warning: A means to release discharge in excess of the infiltration volume safely into the downstream stormwater conveyance system is *REQUIRED*. Freeboard: It is *HIGHLY RECOMMENDED* that two feet of freeboard be provided from the 100-year flood elevation of the infiltration practice to the lowest basement floor elevation of residential, commercial, industrial and institutional buildings located adjacent to the BMP, unless local requirements recommend otherwise.

Drop Structure: Infiltration trenches or subsurface infiltration systems may be designed with a drop structure sized to handle the overflow. This additional volume of stormwater may be directed into the existing stormwater system or it may be diverted to a downstream BMP.

Step 7. Perform ground-water mounding analysis

Groundwater mounding, the process by which a mound of water forms on the water table as a result of recharge at the surface, can be a limiting factor in the design and performance of infiltration practices. A groundwater mounding analysis is RECOMMENDED to verify separation distances required for infiltration practices. For more information on groundwater mounding, see the following sections in this manual.

- Stormwater infiltration and groundwater mounding (http://stormwater.pca.state.mn.us/index.php/Stormwater_infiltration_and_groundwater_mounding)
- When should a mounding analysis be conducted? (http://stormwater.pca.state.mn.us/index.php/Stormwater_infiltration_and_groundwater_mounding#When_should_a _mounding_analysis_be_conducted.3F)
- How to predict the extent of a mound (http://stormwater.pca.state.mn.us/index.php/Stormwater_infiltration_and_groundwater_mounding#How_to_predict_the_extent _of_a_mound)
- Example mound calculations (http://stormwater.pca.state.mn.us/index.php/Stormwater_infiltration_and_groundwater_mounding#Example_mound_calculations)

Step 8. Determine pretreatment volume and design pretreatment measures

See the section on pretreatment (https://stormwater.pca.state.mn.us/index.php?title=Design_criteria_for_infiltration#Pretreatment) for specific pretreatment design guidance

Step 9. Check volume, peak discharge rates and drawdown time against State, local and watershed organization requirements

Follow the design procedures identified in the unified sizing criteria section of the Manual to determine the volume control and peak discharge requirements for water quality, recharge, channel protection, overbank flood and extreme storm.

Perform hand calculations or model the proposed development scenario using a surface water model appropriate for the hydrologic and hydraulic design considerations specific to the site (see also the section on stormwater modeling). This includes defining the parameters of the infiltration practice defined above: elevation and area (defines the storage volume), infiltration rate and method of application (effective infiltration area), and outlet structure and/or flow diversion information. The results of this analysis can be used to determine whether or not the proposed design meets the applicable requirements. If not, the design will have to be re-evaluated (back to Step 5).

The following items are specifically REQUIRED by the MPCA Permit (https://stormwater.pca.state.mn.us/index.php?title=Construction_stormwater_program)

Warning:

- Volume Infiltration or filtration systems shall be sufficient to infiltrate or filter a water quality volume of 1 inch of runoff from the new impervious surfaces created by the project. If this criterion is not met, increase the storage volume of the bioretention practice or treat excess water quality volume (Vwq) in an upstream or downstream BMP (see Step 5). Retrofit and supplemental systems do not need to meet this requirement, provided new impervious surfaces are not created.
- Peak Discharge Rates Since most infiltration systems are not designed for quantity control they generally do not have peak discharge limits. However outflow must
 be limited such that erosion does not occur down gradient.
- Drawdown Infiltration practices shall discharge through the soil or filter media in 48 hours or less. Additional flows that cannot be infiltrated or filtered in 48 hours should be routed to bypass the system through a stabilized discharge point.

Experience has demonstrated that, although the drawdown period is 48 hours, there is often some residual water pooled in the infiltration practice after 48 hours. This residual water may be associated with reduced head, water gathered in depressions within the practice, water trapped by vegetation, and so on. The drawdown period is therefore defined as the time from the high water level in the practice to 1 to 2 inches above the bottom of the facility. This criterion was established to provide the following: wet-dry cycling between rainfall events; unsuitable mosquito breeding habitat; suitable habitat for vegetation; aerobic conditions; and storage for back-to-back precipitation events. This time period has also been called the period of inundation.

Other design requirements may apply to a particular site. The applicant should confirm local design criteria and applicability (see Step 2).

Step 10. Prepare Vegetation and Landscaping Plan

A landscaping plan for an infiltration trench should be prepared to indicate how the enhanced **swale** (https://stormwater.pca.state.mn.us/index.php?title=Dry_swale_(Grass_s wale)) system will be stabilized and established with vegetation. Landscape design should specify proper grass species and wetland plants based on specific site, soils and hydric conditions present along the channel. Further information on plant selection and use occurs in the Minnesota plant lists section.

Step 11. Prepare Operation and Maintenance (O&M) Plan

See Operation and Maintenance section for guidance on preparing an O&M plan.

Step 12. Prepare Cost Estimate

See Cost Considerations section for guidance on preparing a cost estimate that includes both construction and maintenance costs.

Related pages

- Infiltration portal (http://stormwater.pca.state.mn.us/index.php/Infiltration)
- Overview for infiltration
- Types of infiltration
- Design criteria for infiltration
- Construction specifications for infiltration
- Operation and maintenance of stormwater infiltration practices
- Assessing the performance of infiltration
- Calculating credits for infiltration
- Cost-benefit considerations for infiltration
- Case studies for infiltration
- External resources for infiltration
- References for infiltration
- Requirements, recommendations and information for using infiltration basin/underground infiltration BMPs in the MIDS calculator
- Understanding and interpreting soils and soil boring reports for infiltration BMPs

https://stormwater.pca.state.mn.us/index.php?title=Design_criteria_for_infiltration#Step_10._Prepare_Vegetation_and_Landscaping_Plan

- Determining soil infiltration rates
- Cold climate considerations for infiltration practices See [7] (http://stormwater.pca.state.mn.us/index.php/Cold_climate_impact_on_runoff_management#Infiltration), [8] (http://stormwater.pca.state.mn.us/index.pca.state.mn.us/index.pca.state.mn.us

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